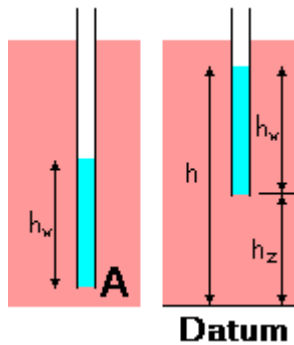


## LECTURE 10

### Permeability Of Soil:

#### Pressure, Elevation and Total Heads

In soils, the interconnected pores provide passage for water. A large number of such flow paths act together, and the average rate of flow is termed the coefficient of permeability, or just permeability. It is a measure of the ease that the soil provides to the flow of water through its pores.



At point **A**, the pore water pressure (**u**) can be measured from the height of water in a standpipe located at that point.

The height of the water column is the **pressure head** ( $h_w$ ).

$$h_w = u / \gamma_w$$

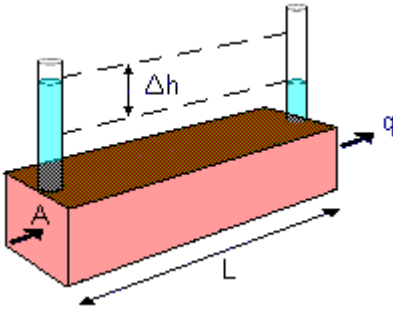
To identify any difference in pore water pressure at different points, it is necessary to eliminate the effect of the points of measurement. With this in view, a datum is required from which locations are measured.

The **elevation head** ( $h_z$ ) of any point is its height above the datum line. The height of water level in the standpipe above the datum is the **piezometric head** (**h**).

$$h = h_z + h_w$$

**Total head** consists of *three components*: elevation head, pressure head, and velocity head. As seepage velocity in soils is normally low, velocity head is ignored, and total head becomes equal to the piezometric head. Due to the low seepage velocity and small size of pores, the flow of water in the pores is steady and laminar in most cases. Water flow takes place between two points in soil due to the difference in total heads.

Darcy's law states that there is a linear relationship between flow velocity (**v**) and hydraulic gradient (**i**) for any given saturated soil under steady laminar flow conditions.



If the rate of flow is **q** (volume/time) through cross-sectional area (**A**) of the soil mass, Darcy's Law can be expressed as

$$v = q/A = k.i$$

where **k** = permeability of the soil

$$i = \Delta h/L$$

**Δh** = difference in total heads

**L** = length of the soil mass

The flow velocity (**v**) is also called the Darcian velocity or the **superficial velocity**. It is different from the actual velocity inside the soil pores, which is known as the **seepage velocity, v<sub>s</sub>**. At the particulate level, the water follows a tortuous path through the pores. Seepage velocity is always greater than the superficial velocity, and it is expressed as:

$$v_s = \frac{q}{A_v} = \frac{q}{A_v} \cdot \frac{A}{A} \approx \frac{v}{n}$$

where **A<sub>v</sub>** = Area of voids on a cross section normal to the direction of flow

**n** = porosity of the soil

### Permeability of Different soils:

Permeability (**k**) is an engineering property of soils and is a function of the soil type. Its value depends on the average size of the pores and is related to the distribution of particle sizes, particle shape and soil structure. The ratio of permeabilities of typical sands/gravels to those of typical clays is of the order of **10<sup>6</sup>**. A small proportion of fine material in a coarse-grained soil can lead to a significant reduction in permeability.

For different soil types as per grain size, the orders of magnitude for permeability are as follows:

Soil	k (cm/sec)
Gravel	10 <sup>0</sup>
Coarse sand	10 <sup>0</sup> to 10 <sup>-1</sup>
Medium sand	10 <sup>-1</sup> to 10 <sup>-2</sup>

Fine sand	$10^{-2}$ to $10^{-3}$
Silty sand	$10^{-3}$ to $10^{-4}$
Silt	$1 \times 10^{-5}$
Clay	$10^{-7}$ to $10^{-9}$

In soils, the permeant or pore fluid is mostly water whose variation in property is generally very less. Permeability of all soils is strongly influenced by the density of packing of the soil particles, which can be represented by void ratio (**e**) or porosity (**n**).

### For Sands

In sands, permeability can be empirically related to the square of some representative grain size from its grain-size distribution. For filter sands, Allen Hazen in 1911 found that  $k \propto 100 (D_{10})^2$  cm/s where  $D_{10}$ = effective grain size in cm.

Different relationships have been attempted relating void ratio and permeability, such as  $k \propto e^3/(1+e)$ , and  $k \propto e^2$ . They have been obtained from the Kozeny-Carman equation for laminar flow in saturated soils.

$$k = \frac{1}{k_0 k_T S_s^2} \cdot \frac{e^3}{1+e} \cdot \frac{\gamma_w}{\eta}$$

where  $k_0$  and  $k_T$  are factors depending on the shape and tortuosity of the pores respectively,  $S_s$  is the surface area of the solid particles per unit volume of solid material, and  $\gamma_w$  and  $\eta$  are unit weight and viscosity of the pore water. The equation can be reduced to a simpler form as

$$k = C \cdot \frac{e^3}{1+e} \approx C \cdot e^2$$

### For Silts and Clays

For silts and clays, the Kozeny-Carman equation does not work well, and **log k** versus **e** plot has been found to indicate a linear relationship.

For clays, it is typically found that

$$\log_{10} k = \frac{e - e_k}{C_k}$$

where  $C_k$  is the permeability change index and  $e_k$  is a reference void ratio.